

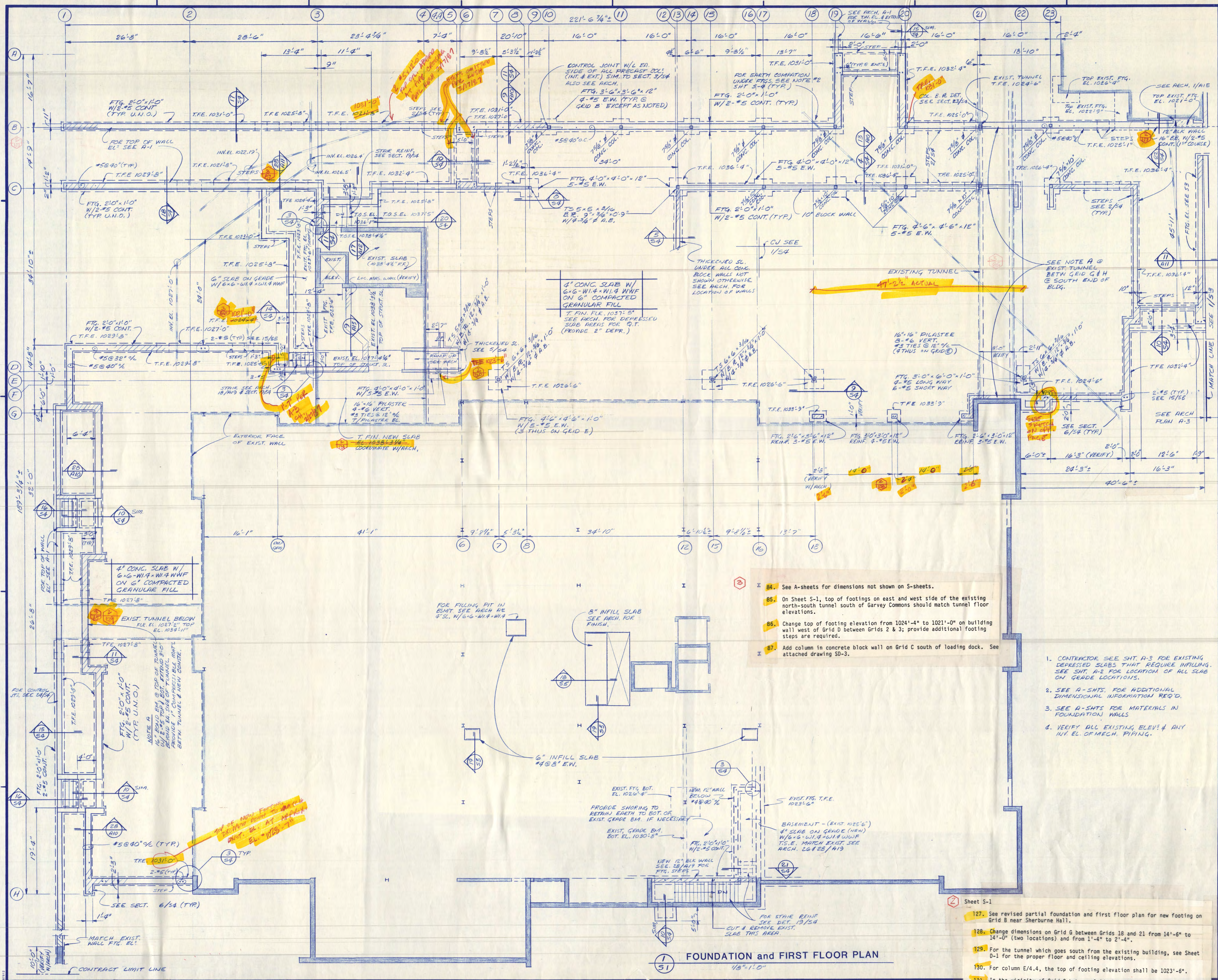
ROBERT RIPE & ASSOCIATES INC.
FOODSERVICE CONSULTANTS
10249 Yellow Circle Drive Suite 200
Minneapolis, MN 55343 (612) 935-2334

I HEREBY CERTIFY THAT THIS PLAN SPECIFICATION OR REPORT WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION AND THAT I AM DULY REGISTERED ARCHITECT/ENGINEER UNDER THE LAWS OF THE STATE OF MINNESOTA.

DATE _____ REG. NO. _____
FREDRIC WEMLINGER AND ASSOCIATES INC.
ARCHITECTS AND ENGINEERS
SAINT CLOUD, MINNESOTA 56301
TELEPHONE 812-253-2100

GARVEY COMMONS/ SHERBURNE HALL ADDITIONS & REMODELING
ST. CLOUD STATE UNIVERSITY
ST. CLOUD MINNESOTA

REVISED PROJECT NO. 86015
ISSUE DATE 12-15-86
DRAWN BY
CHECKED
APPROVED ARCHITECT
SCALE NO SCALE



FOUNDATION and FIRST FLOOR PLAN

- 84. See A-sheets for dimensions not shown on S-sheets.
- 85. On Sheet S-1, top of footings on east and west side of the existing north-south tunnel south of Garvey Commons should match tunnel floor elevations.
- 86. Change top of footing elevation from 1024'-4" to 1021'-0" on building wall west of Grid D between Grids 2 & 3; provide additional footing steps as required.
- 87. Add column in concrete block wall on Grid C south of loading dock. See attached drawing SD-3.

1. CONTRACTOR SEE SHT. A-3 FOR EXISTING DEPRESSED SLABS THAT REQUIRE INFILLING. SEE SHT. A-2 FOR LOCATION OF ALL SLAB ON GRADE LOCATIONS.
2. SEE A-SHTS. FOR ADDITIONAL DIMENSIONAL INFORMATION REQ'D.
3. SEE A-SHTS FOR MATERIALS IN FOUNDATION WALLS
4. VERIFY ALL EXISTING ELEV'S & ANY INV. EL. OF MECH. PIPING.

WAYNE C. LARSON
CONSULTING ENGINEERS
2129 SECOND STREET WHITE BEAR, MN 55110
612/429-8363

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**GARVEY COMMONS/
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	SCALE 1/8"=1'-0"

Sheet S-1

127. See revised partial foundation and first floor plan for new footing on Grid B near Sherburne Hall.

128. Change dimensions on Grid G between Grids 18 and 21 from 14'-6" to 14'-0" (two locations) and from 1'-4" to 2'-4".

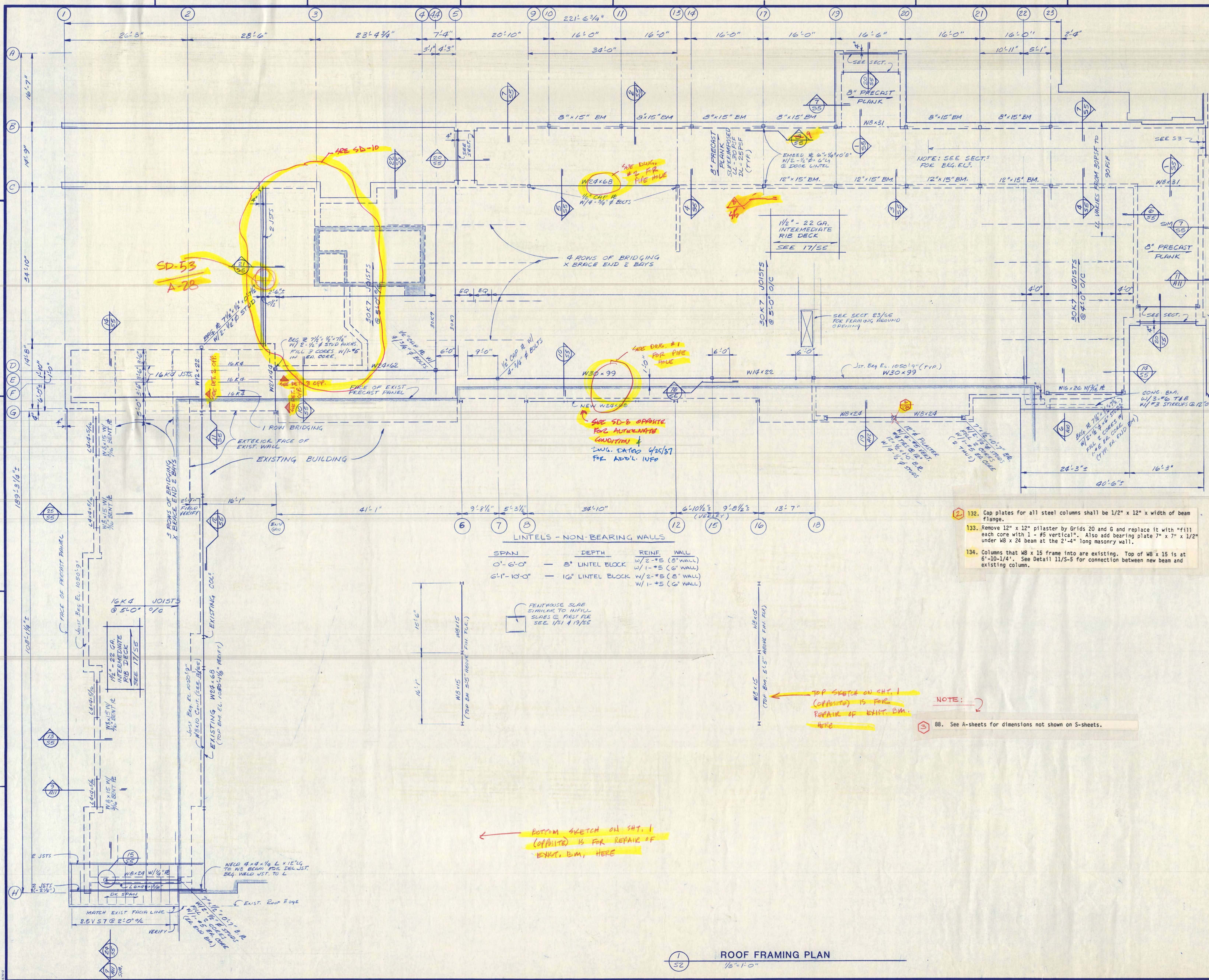
129. For the tunnel which goes south from the existing building, see Sheet D-1 for the proper floor and ceiling elevations.

130. For column E/4.4, the top of footing elevation shall be 1023'-6".

131. In the vicinity of Grid G between Grids 3 and 4, delete the elevation of 1038'-3-1/4" as the top finish new slab; see Sheet A-3 for correct elevations.



S-1



SPAN	DEPTH	REINF.	WALL
0'-6"-0"	8" LINTEL BLOCK	w/2-#5 (6" WALL) w/1-#5 (6" WALL)	
6'-1"-0"	16" LINTEL BLOCK	w/2-#5 (6" WALL) w/1-#5 (6" WALL)	

- 132. Cap plates for all steel columns shall be 1/2" x 12" x width of beam flange.
- 133. Remove 12" x 12" pilaster by Grids 20 and G and replace it with "fill" each core with 1-#5 vertical". Also add bearing plate 7" x 7" x 1/2" under W8 x 24 beam at the 2'-4" long masonry wall.
- 134. Columns that W8 x 15 frame into are existing. Top of W8 x 15 is at 6'-10-1/4". See Detail 11/S-5 for connection between new beam and existing column.

88. See A-sheets for dimensions not shown on S-sheets.

← BOTTOM SKETCH ON SHT. 1 (OPPOSITE) IS FOR REPAIR OF EXIST. BM, HERE

← TOP SKETCH ON SHT. 1 (OPPOSITE) IS FOR REPAIR OF EXIST. BM, HERE

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DATE 12-15-86 REG. NO. 13935

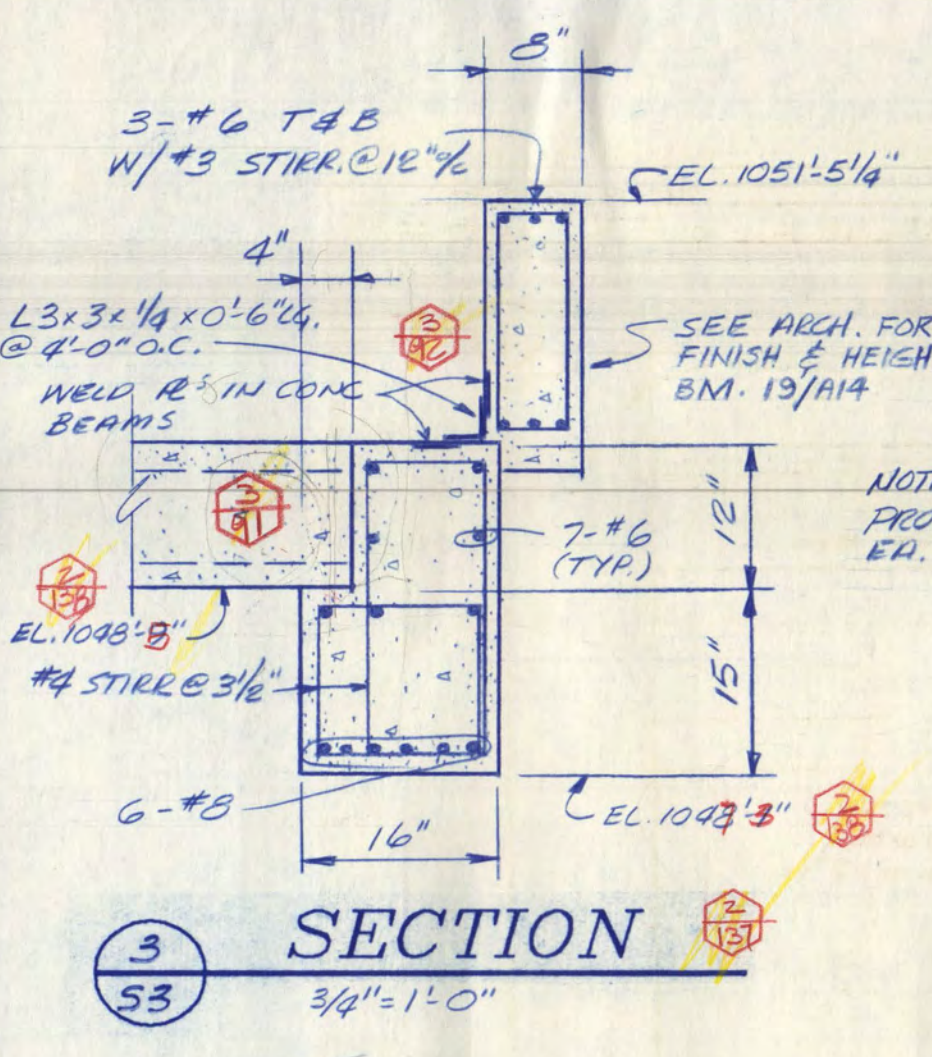
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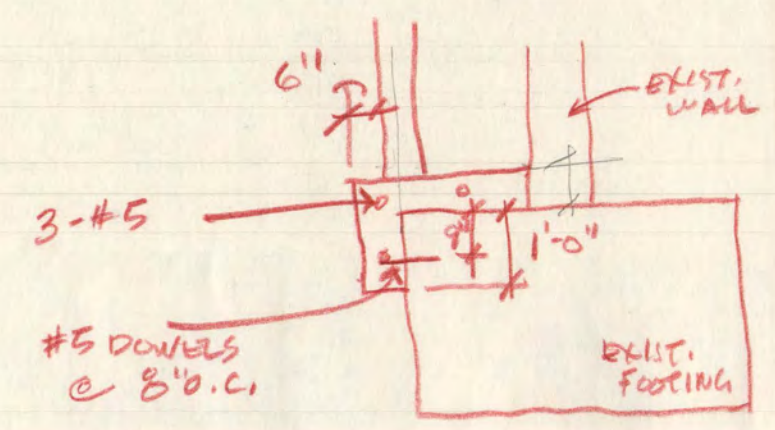
NORTH → **S-2**

1
ROOF FRAMING PLAN
1/8"=1'-0"



NOTE: PROVIDE 8" BRG. ED. ENO OF BHS.

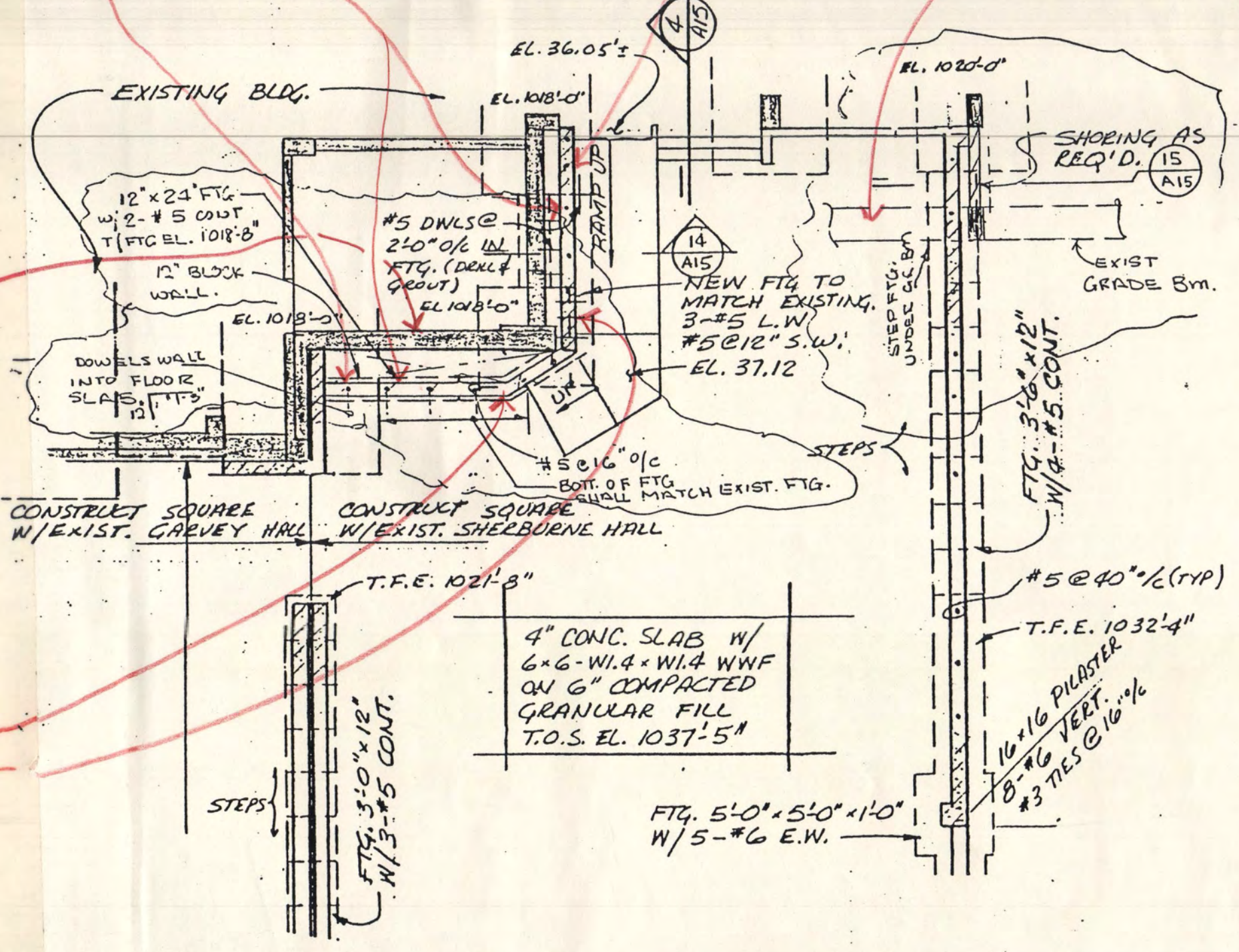
SEE DETAIL 4 FOR REPAIR OF EXIST. CONC. REM. CUT IN THIS AREA



THESE SECTIONS OF WALL BUILT THUS!

THESE SECTIONS OF NEW WALL SIT ON EXIST. COL. PADS. THESE SECTIONS OF NEW WALL HAVE 10" SAND BEAM W/ 4-#5 SETI ON SOIL (NO FOOTING)

AS1 #2 ANCHORS REMOVAL OF THIS SECTION - SOUTH END OF REMAINING GRADE BEAM TO REMAIN ON NORTH WALL OF ADD'D.

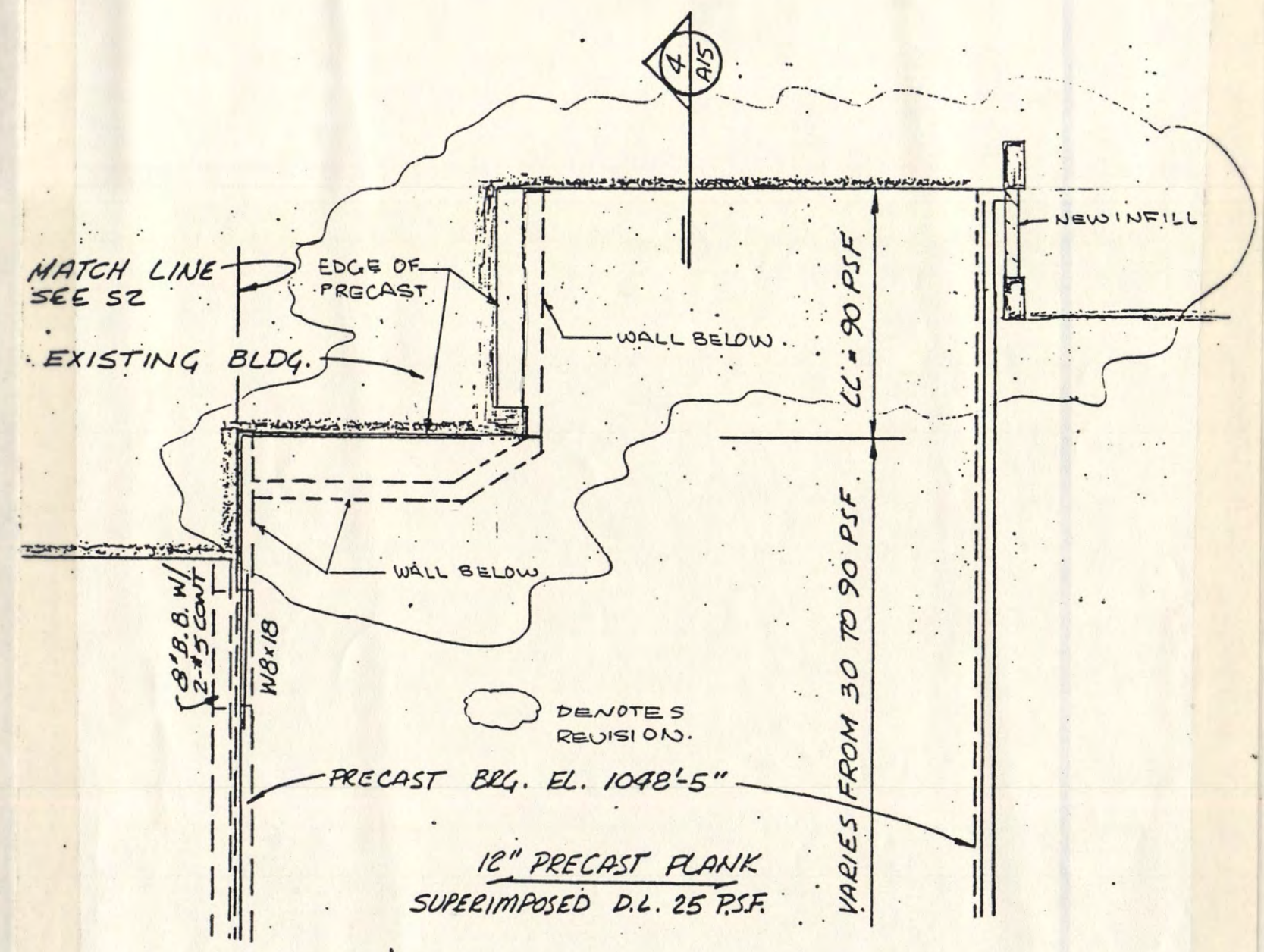


PARTIAL FOUND. & FLOOR PLAN - SHERBURNE HALL SCALE 1/8" = 1'-0"

ADDENDUM #2, 17 JANUARY 1987, PROJECT 86015
 WAYNE C. LARSON
 STRUCTURAL ENGINEERS
 2129 2ND STREET
 WHITE BEAR, MN 55110
 612/429-8363

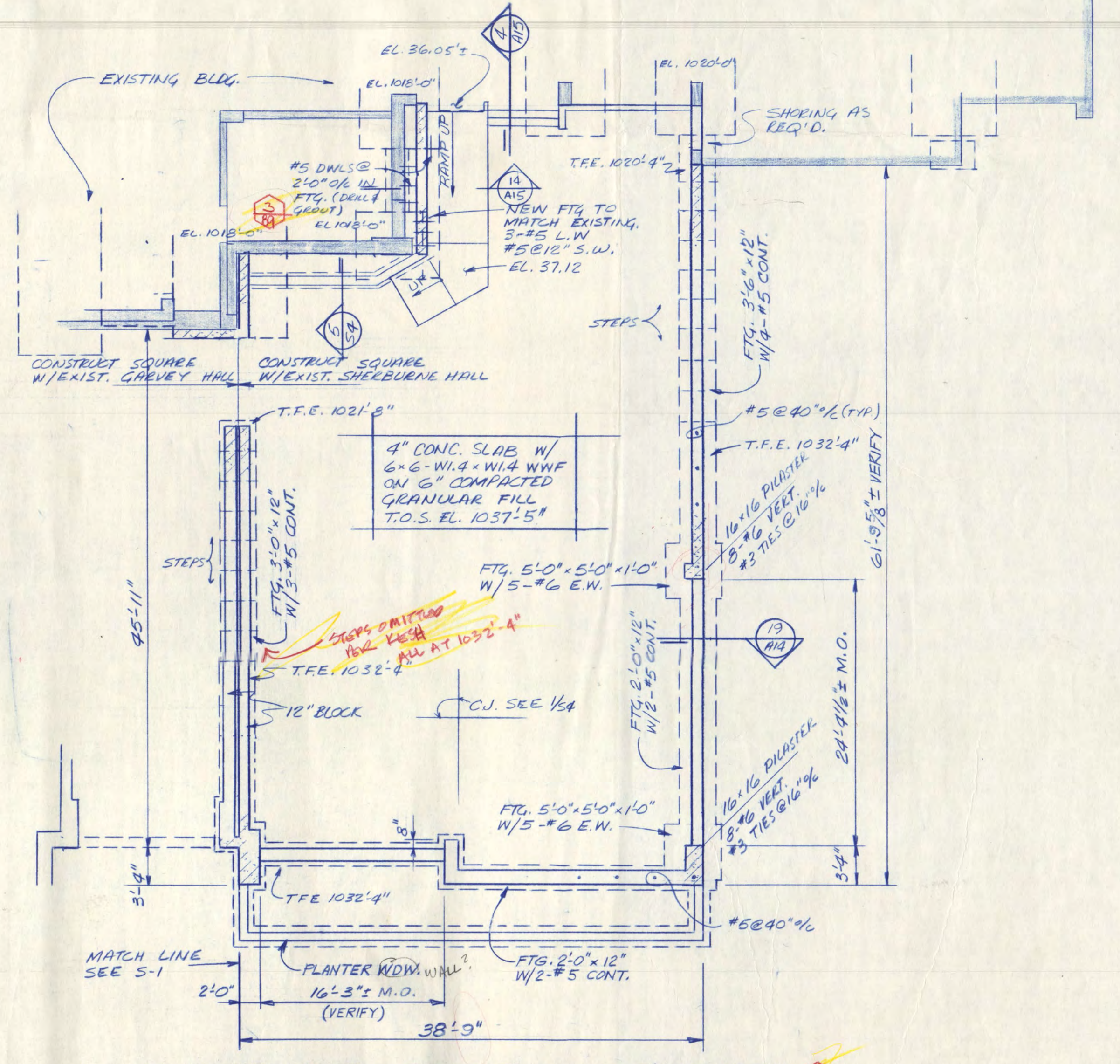
- 135. See revised "partial foundation and first floor plan" and "partial roof plan".
- 136. Add note on roof plan. "See mechanical and architectural drawings for locations of roof opening".
- 137. Details 1 and 3: See A-sheets for proper location of pilasters and concrete beam at north wall window opening.
- 138. Detail 3: Change the bottom of beam elevation to 1047'-3" and precast plank bearing elevation to 1048'-5".

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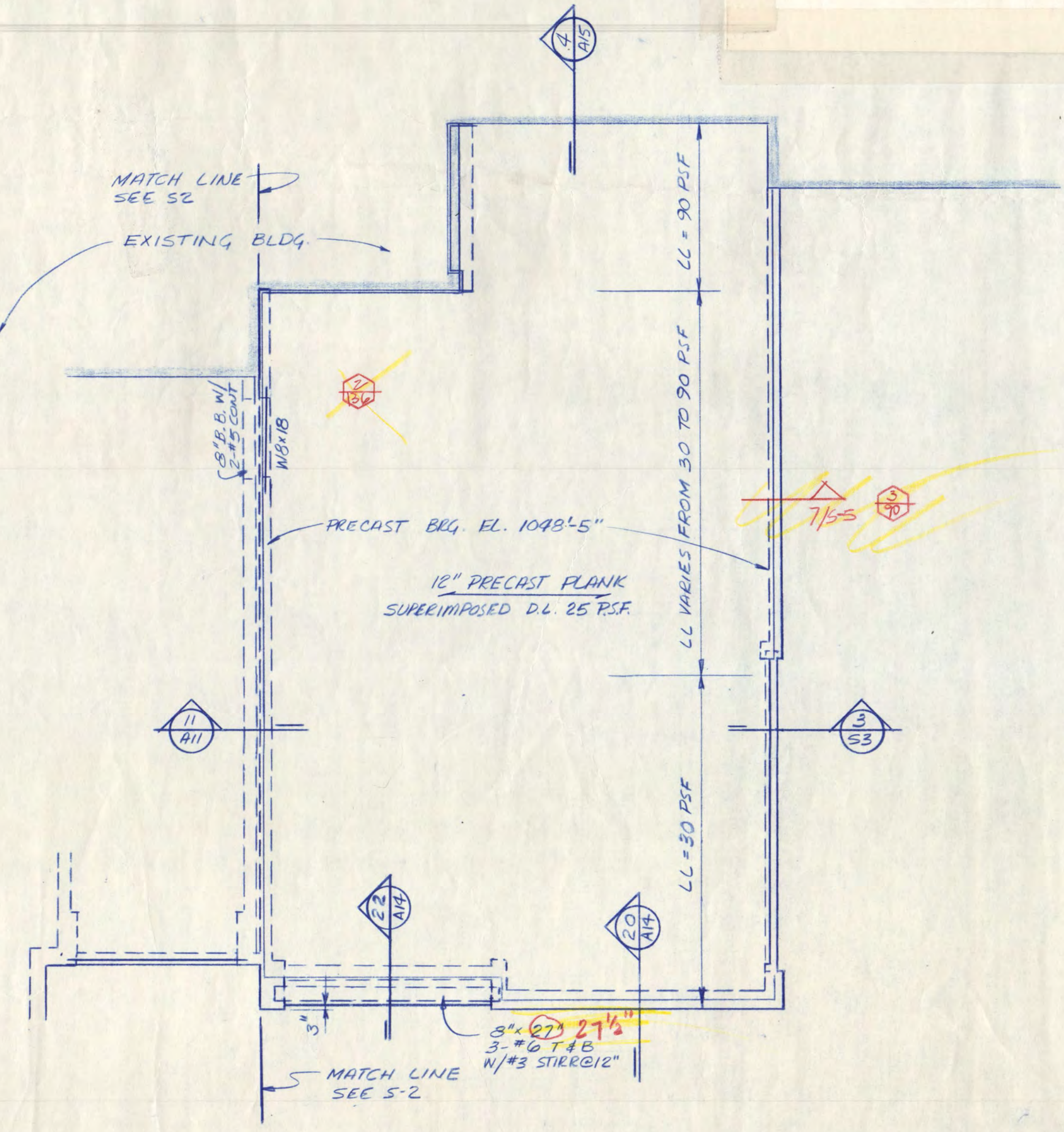


PARTIAL ROOF PLAN - SHERBURNE HALL SCALE 1/8" = 1'-0"

- 89. Detail 1: Connections between new and existing walls is per detail on 3/5-4.
 - 90. Detail 2: Add section cut 7/5-5 on north wall.
 - 91. Detail 3: Connection between precast plank and concrete beam is similar to Detail 3/5-5.
 - 92. Detail 3: All weld plates are 4" x 4" x 1/2" plate with two 5/8" diam. x 6" studs.
- Weld plates and connecting angles are by Section 0550; installed by Section 0330.



FOUNDATION & FLOOR PLAN - LOUNGE AREA SCALE 1/8" = 1'-0"



ROOF PLAN - LOUNGE AREA SCALE 1/8" = 1'-0"

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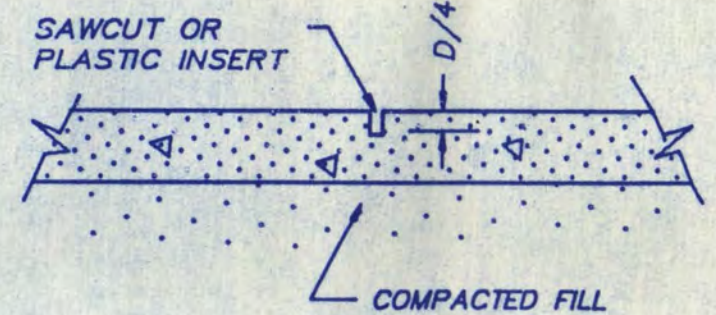
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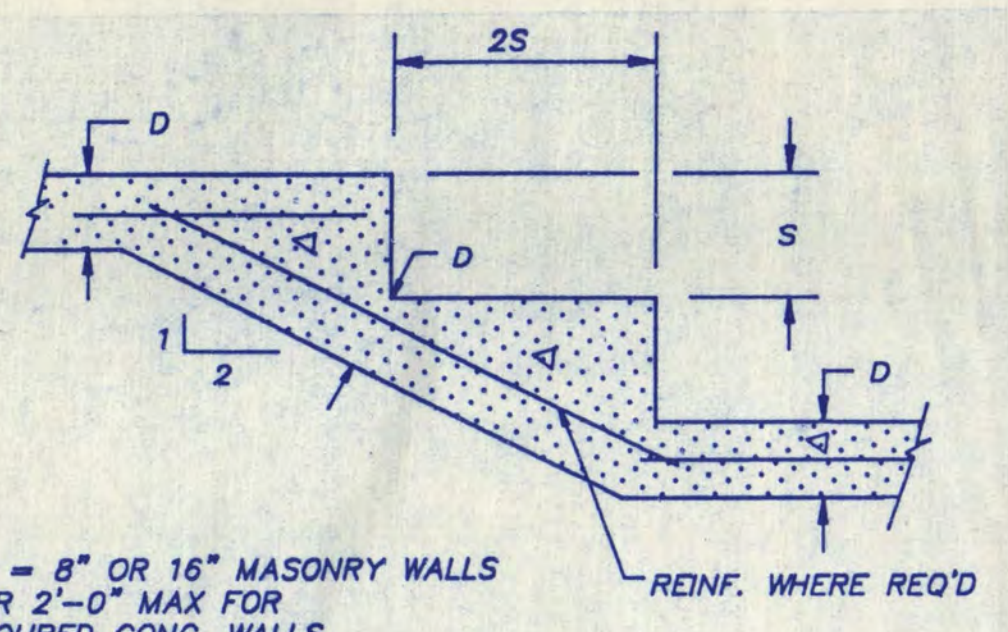
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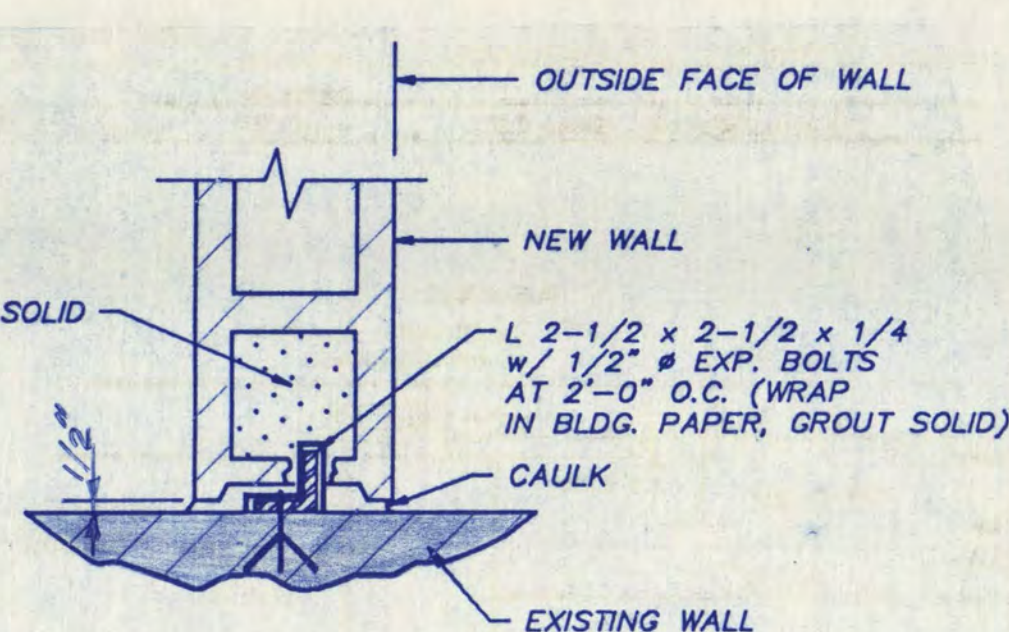
1. SAW CUT CONTROL JOINTS TO BE CUT 4 TO 12 HRS. AFTER CONCRETE HARDENS
2. CONTROL JOINT LOCATIONS TO BE COORDINATED WITH ARCH. PLANS SO THAT JOINTS FALL AT WALL LOCATIONS WHERE POSSIBLE
3. FOR SPACING OF CONTROL JOINTS SEE STRUCTURAL NOTES.

SECTION 1
NO SCALE

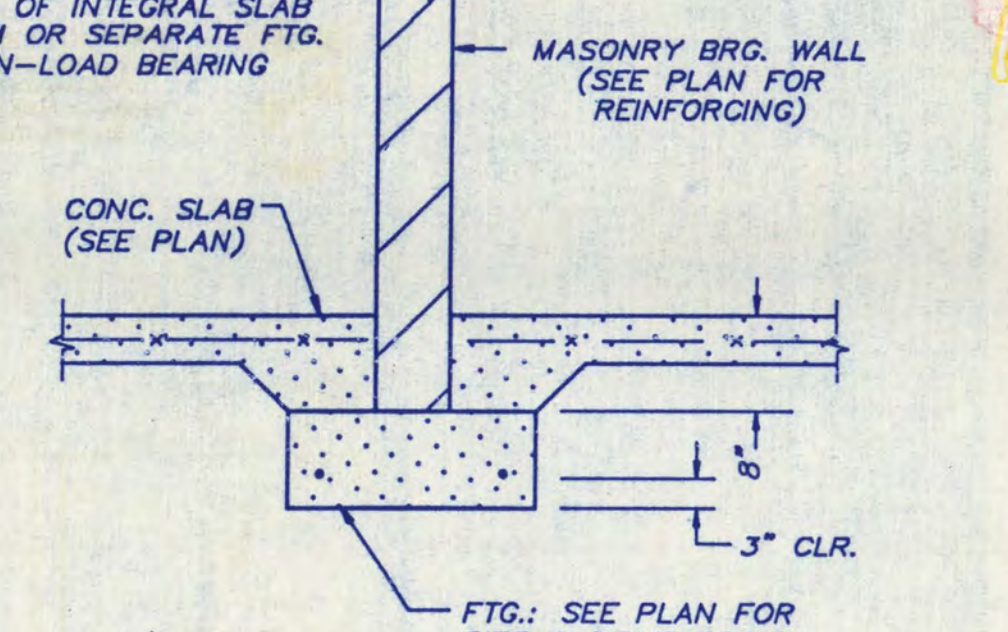


5 = 8" OR 16" MASONRY WALLS OR 2'-0" MAX FOR POURED CONC. WALLS OR PRECAST.

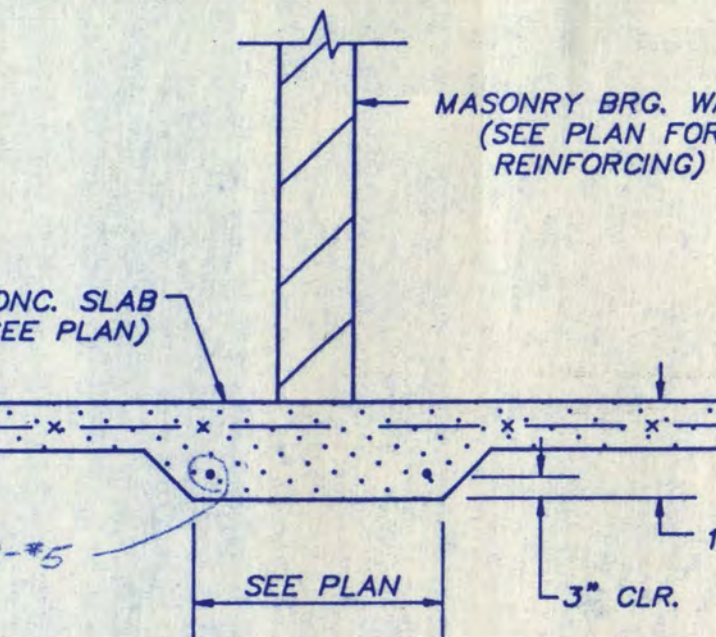
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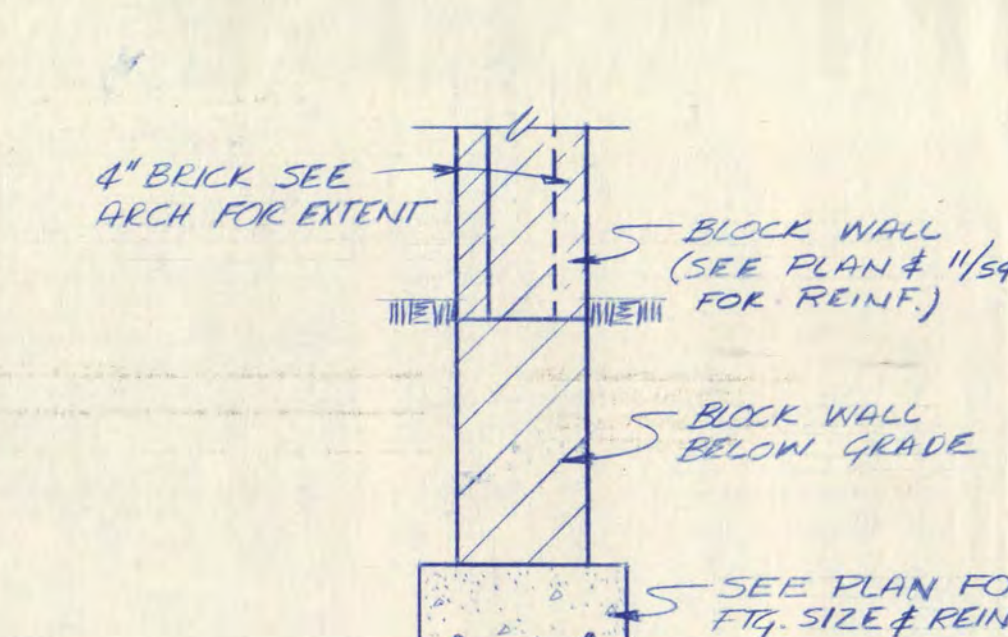
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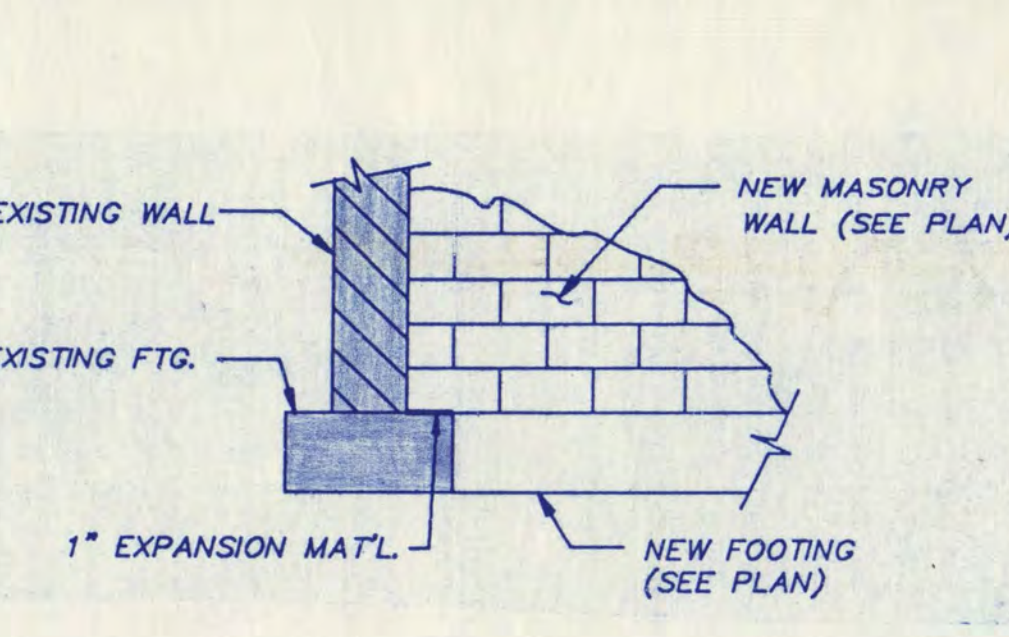
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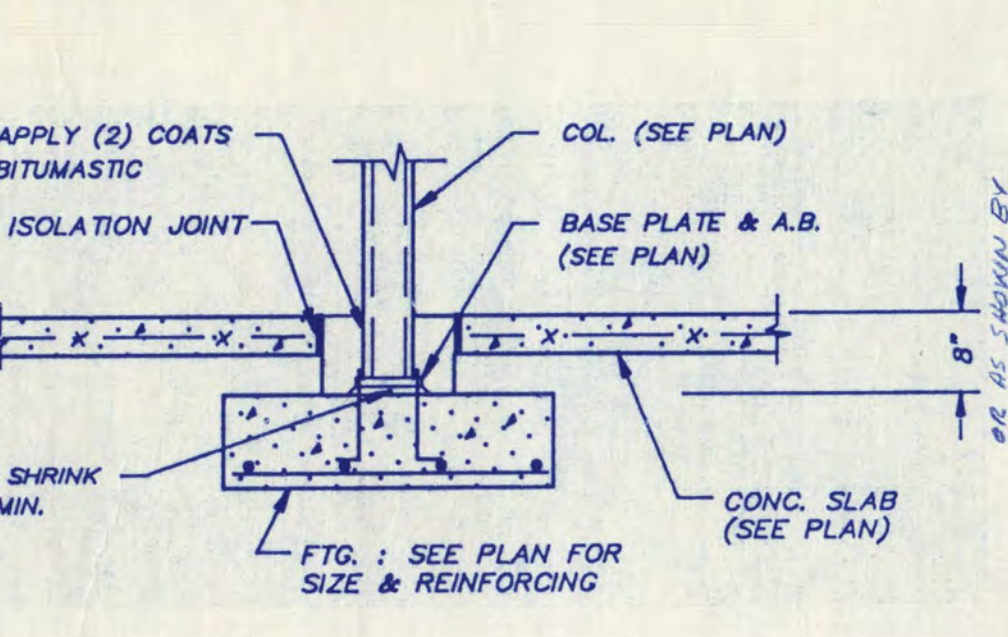
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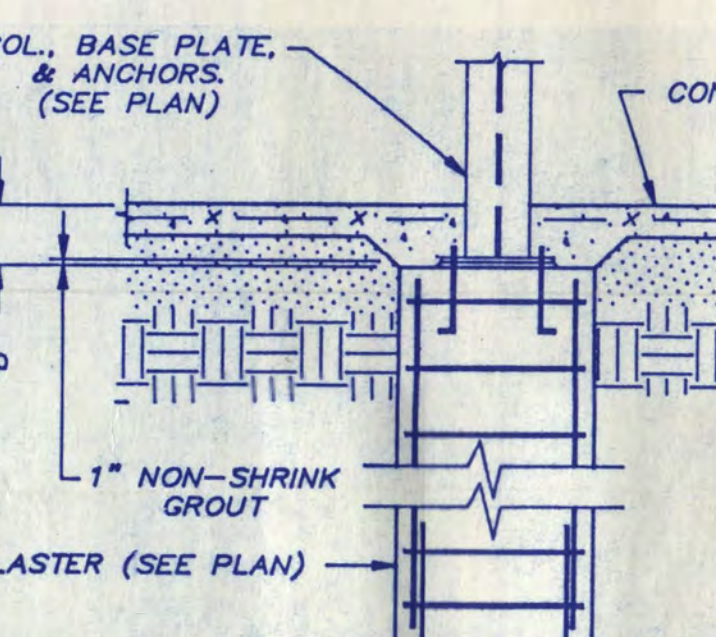
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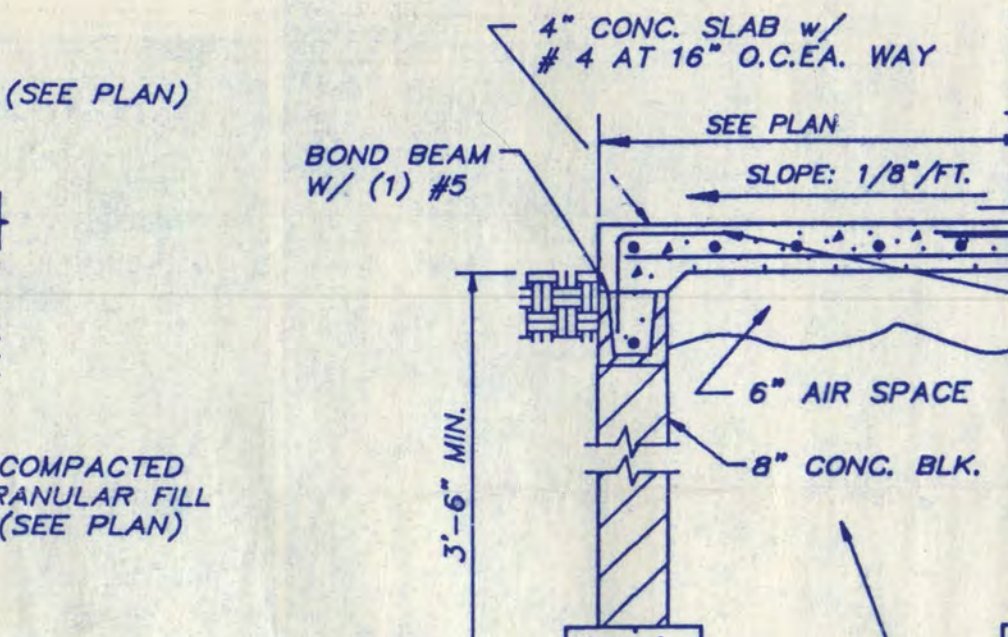
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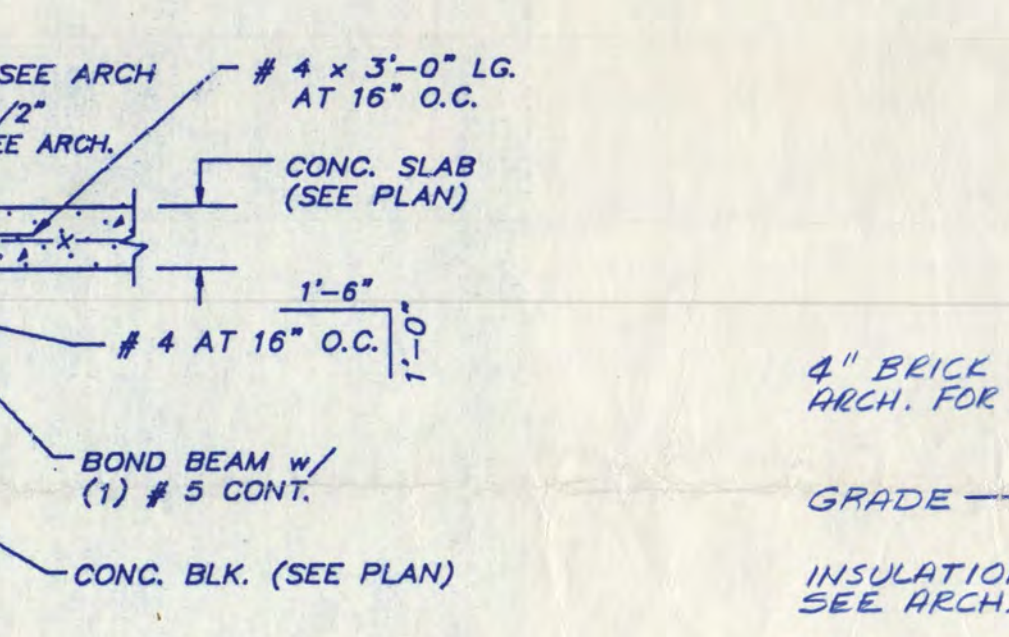
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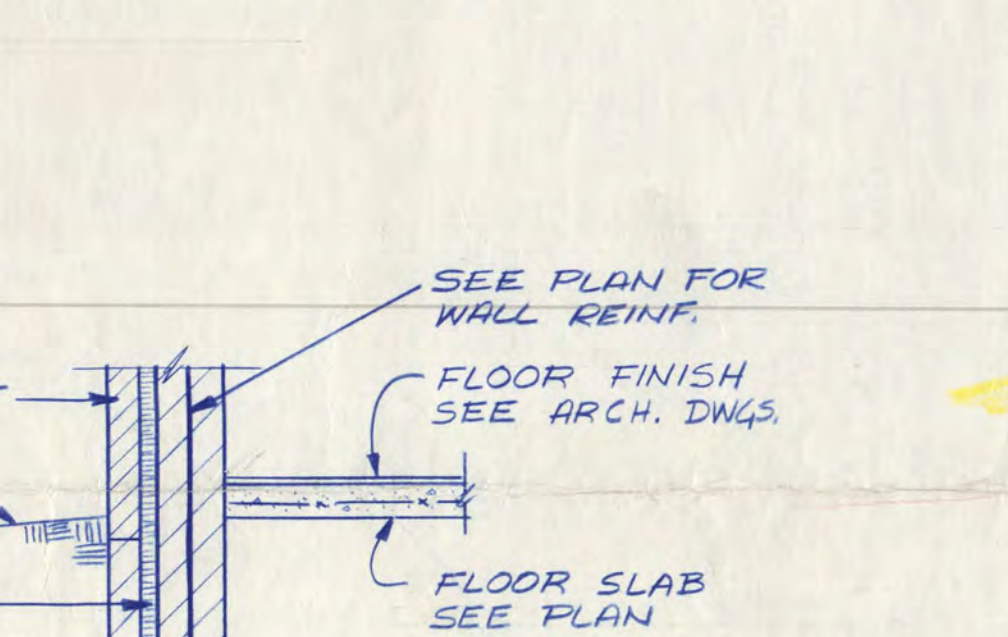
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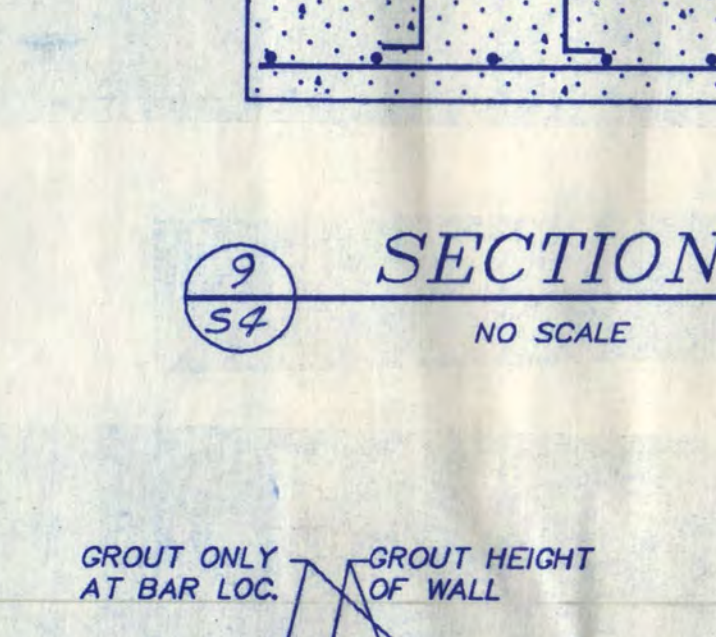
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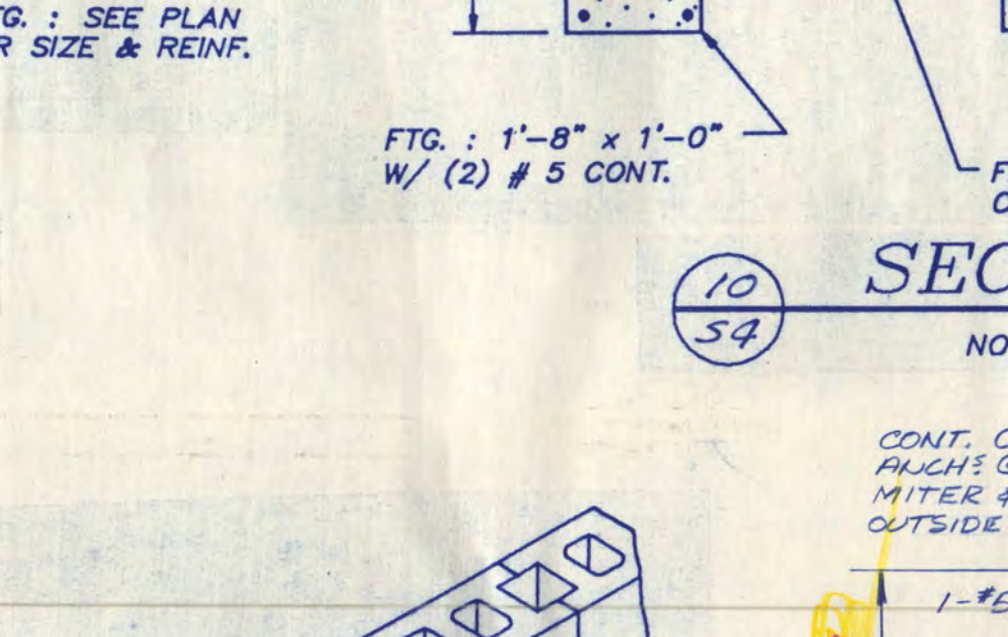
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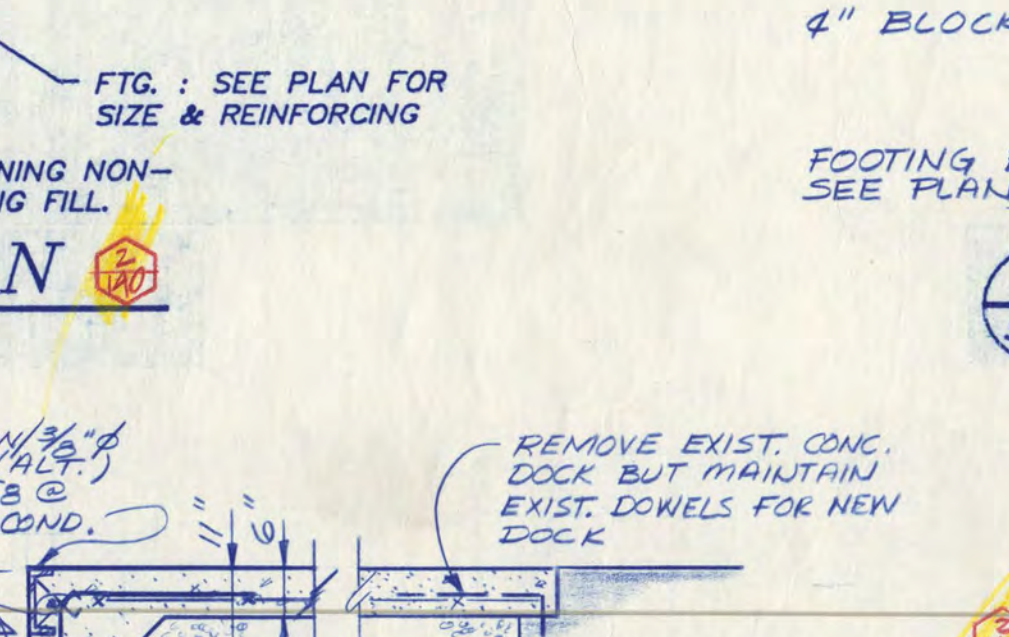
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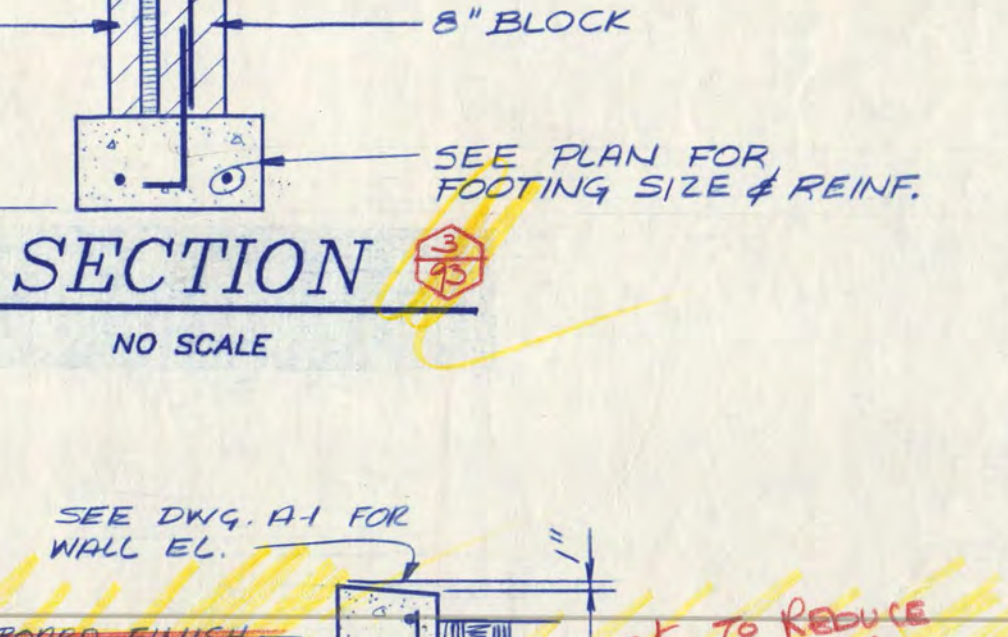
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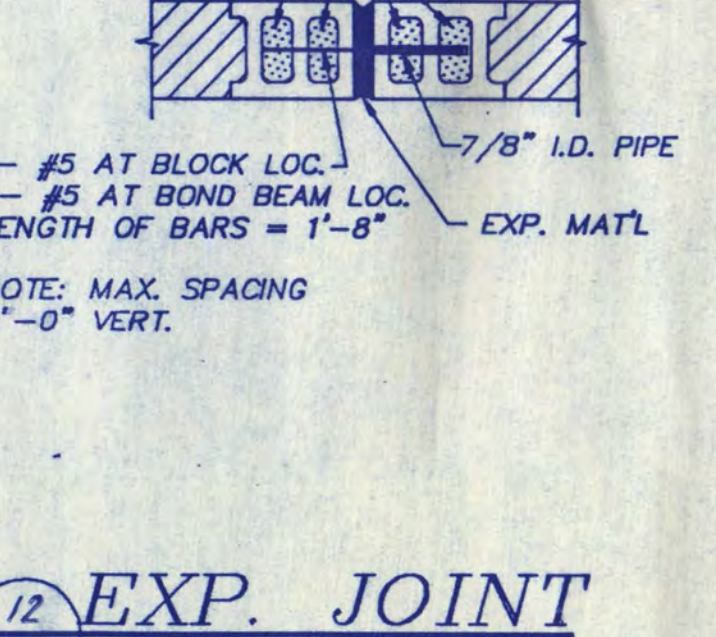
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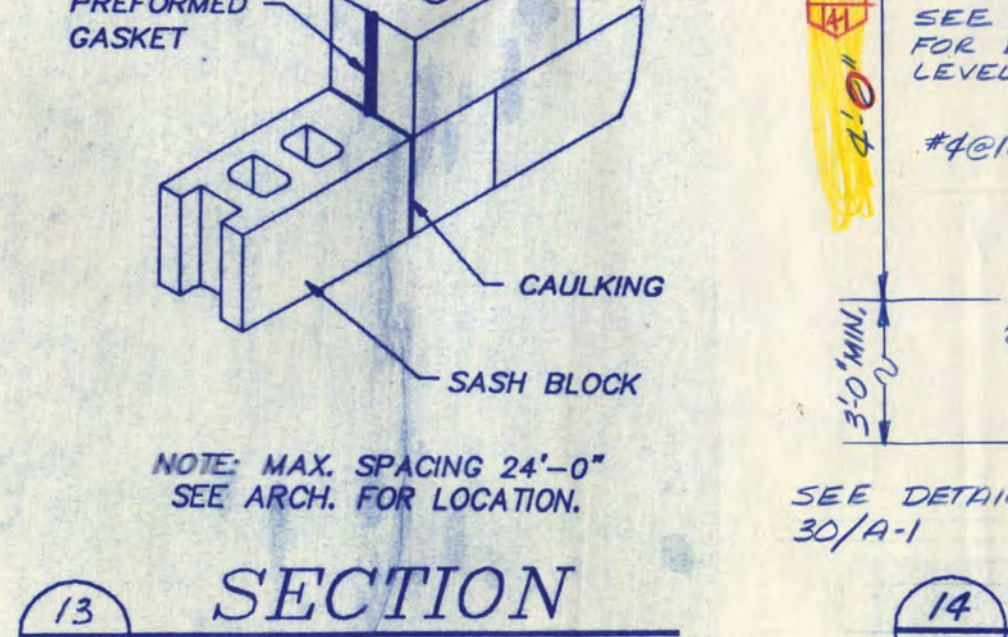
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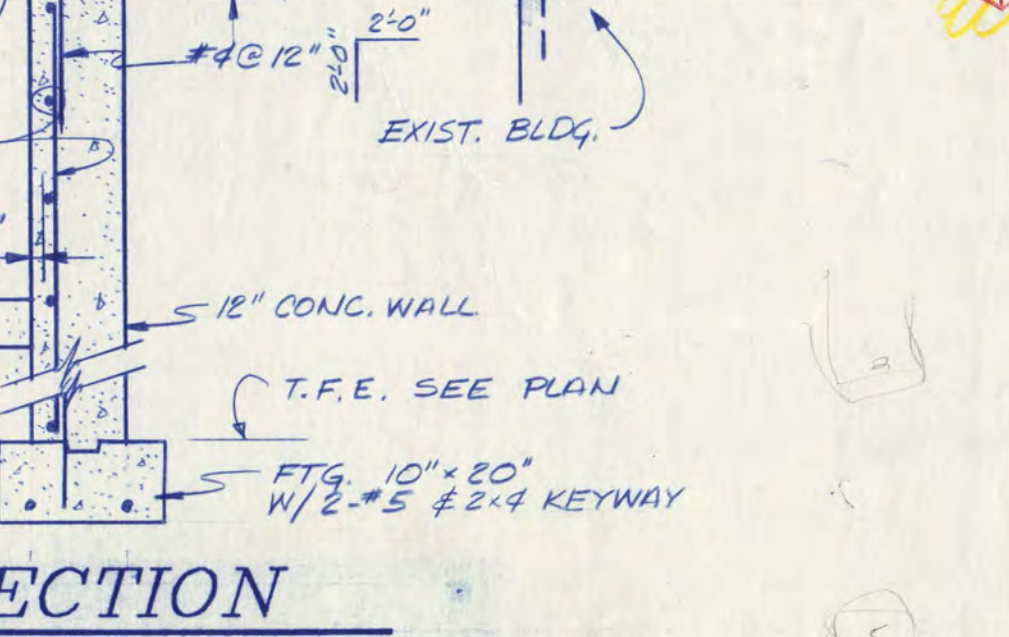
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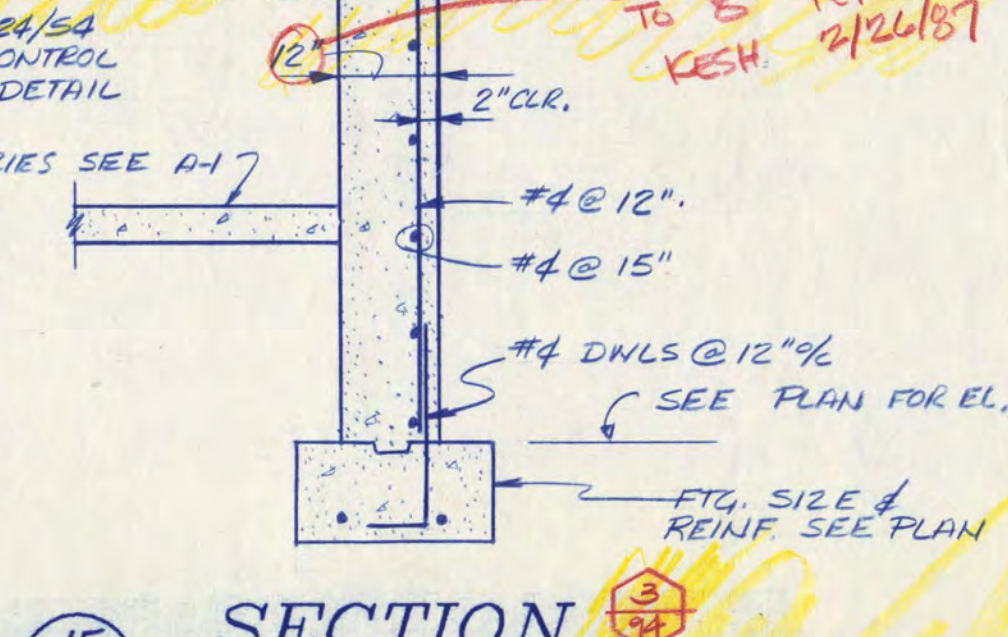
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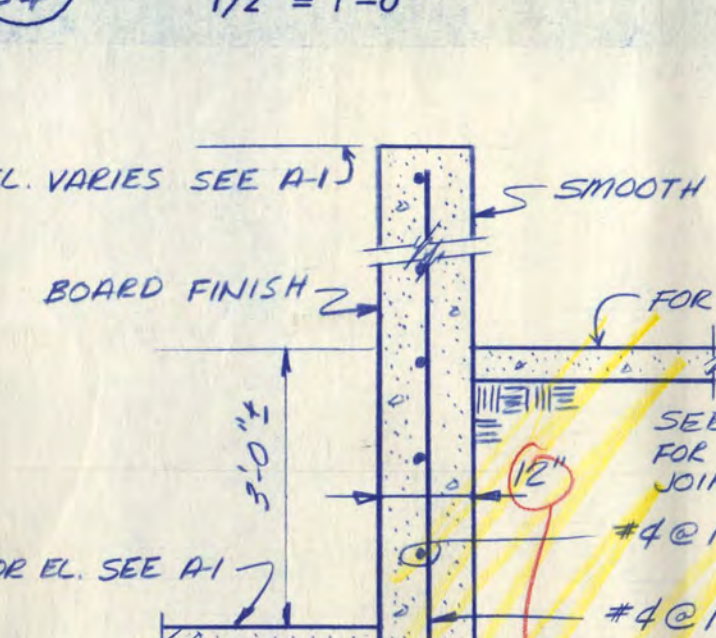
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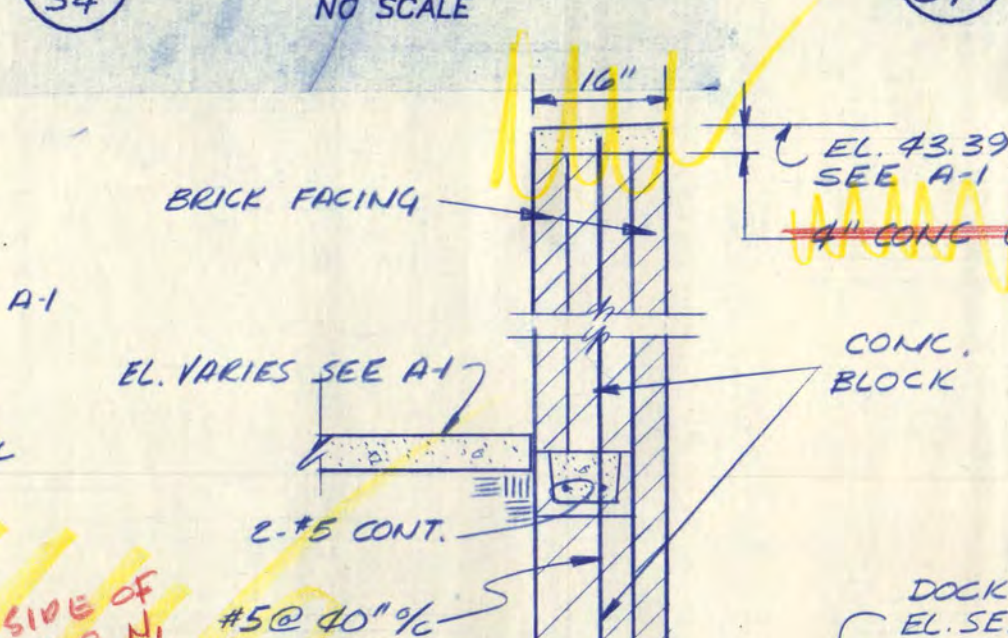
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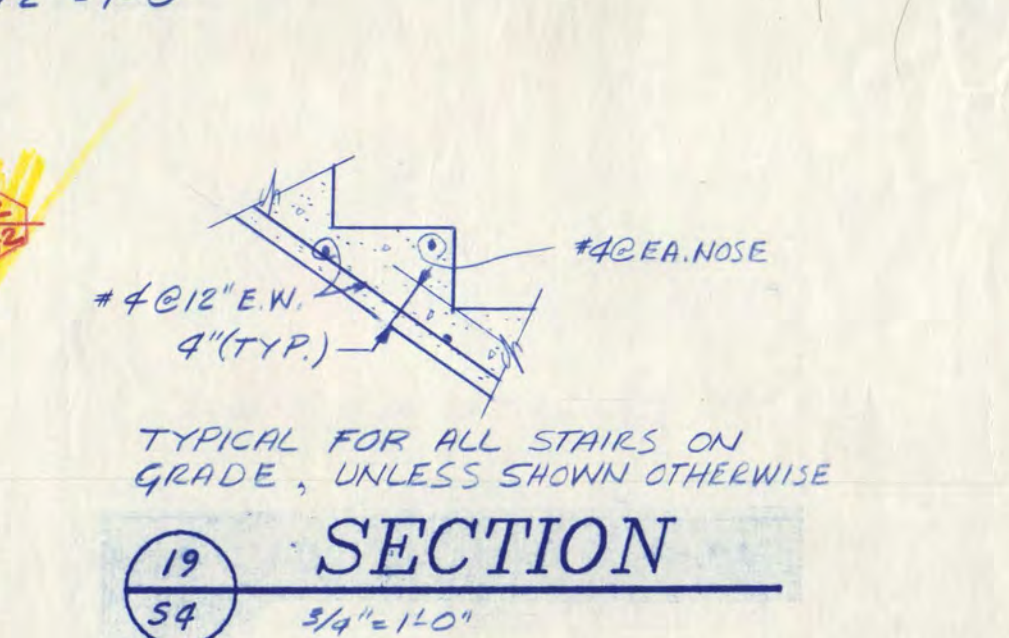
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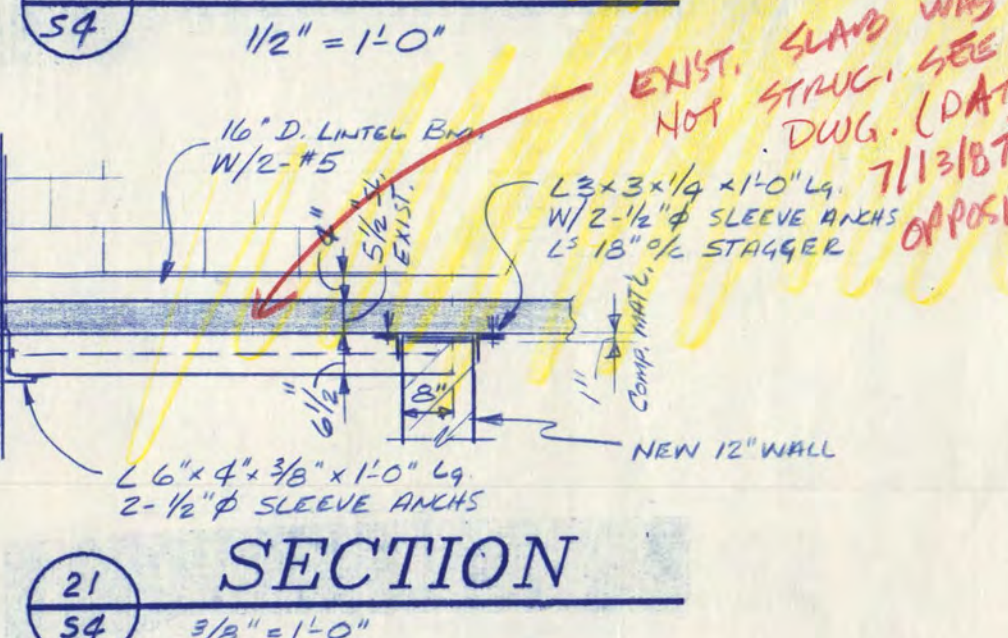
SECTION 21
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SECTION 22
NO SCALE



SECTION 23
NO SCALE



SECTION 24
NO SCALE

- GENERAL STRUCTURAL NOTES**
- BUILDING CODES USED FOR DESIGN:**
 - MINNESOTA BUILDING CODE, 1985 EDITION.
 - FOUNDATIONS:**
 - ALL FOOTINGS SHALL BEAR ON NATURAL UNDISTURBED SOIL OR ON COMPACTED GRANULAR FILL (SEE SPEC. OR SOIL REPORT). AT THE TIME OF EXCAVATION FOOTING ELEVATIONS SHOWN SHALL BE OPEN TO INSPECTION BY THE ENGINEER. GRANULAR FILL SHALL BE COMPACTED TO 98% STANDARD DENSITY (ASTM D698-79).
 - FOOTINGS SHALL BE STRIP TYPE UNLESS OTHERWISE NOTED.
 - SQUARE FOOTING REINFORCEMENT SHALL BE EACH WAY WITH EQUAL SPACING UNLESS OTHERWISE NOTED.
 - IF SOIL AT BOTTOM OF FOOTINGS AS DETAILED IS OF QUESTIONABLE BEARING VALUE, THE ARCHITECT'S OFFICE SHALL BE NOTIFIED AT ONCE.
 - WALL FOOTINGS TO BE STEPPED WHERE ELEVATIONS CHANGE: 1 VERTICAL TO 2 HORIZONTAL IN VERTICAL INCREMENTS OF 1'-4" (MAXIMUM), UNLESS OTHERWISE NOTED.
 - ALL EXTERIOR WALL FOOTINGS SHALL HAVE A MINIMUM SOIL COVER OF 1'-6" MEASURED FROM BOTTOM OF FOOTING UNLESS OTHERWISE NOTED.
 - DESIGN LOADS:**
 - DESIGN LIVE LOADS: ROOFS: 20 PSF; FLOORS: 40 PSF; PLUSES DRIFTING AND/OR SLIDING SNOW: ***30 PSF
 - DESIGN STRESSES:**
 - CONCRETE: 3000 STD. WT. INTERIOR SLABS; 4000 STD. WT. FOOTINGS; 4000 STD. WT. COLUMNS & CONC. BEAMS; 4000 STD. WT. AIR-ENTRAINED EXTERIOR SLABS
 - REINFORCEMENT: F_y = 60,000 PSI
 - STRUCTURAL STEEL: F_y = 36,000 PSI
 - STRUCTURAL TUBING: F_y = 45,000 PSI
 - STANDARD STEEL PIPE: F_y = 36,000 PSI
 - CONCRETE COVERAGE FOR REINFORCEMENT:**
 - FOOTINGS: 3"
 - EXPOSED EXTERIOR CONCRETE: 2"
 - SLAB ON GRADE: CENTER OF SLAB
 - MASONRY COVERAGE FOR REINFORCEMENT:**
 - WALLS: 1 1/2" TO TIES
 - PISTERS: 3" TO TIES
 - BEAMS: 1 1/2" TO STRUTS
 - REINFORCING STEEL:**
 - THE REINFORCING STEEL CONTRACTOR SHALL FABRICATE ALL REINFORCEMENT AND FURNISH ALL ACCESSORIES, CHAIRS, SPACER BARS AND SUPPORTS NECESSARY TO SECURE THE REINFORCEMENT UNLESS SHOWN OTHERWISE ON THE PLANS AND/OR DETAILS.
 - REINFORCING STEEL SHALL BE ASTM A615, GRADE 60.
 - WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185.
 - CONCRETE REINFORCEMENT SHALL BE PLACED ACCORDING TO THE CRSI "RECOMMENDED PRACTICE FOR PLACING REINFORCING BARS".
 - COMPRESSION AND TENSION LAP SPICES SHALL BE 36 BAR DIAMETER MINIMUM UNLESS OTHERWISE NOTED.
 - TOP BARS SHALL BE HOOKED AT END SPANS.
 - HORIZONTAL REINFORCING STEEL IN FOOTINGS AND CONCRETE WALLS SHALL BE CONTINUOUS AROUND CORNERS.
 - REINFORCED MASONRY:**
 - GROUP FOR VERTICALLY REINFORCED MASONRY WALLS AND BOND BEAMS SHALL CONSIST OF 1 PORTLAND CEMENT, 1 1/2 FINE AGGREGATE, 2 FEA GRAVEL, F_c = 3,000 P.S.I., & 28 DAYS. GROUT SLUMP SHALL BE 3" TO 10".
 - ALL MASONRY WALLS SHALL HAVE HORIZONTAL REINFORCING CONSISTING OF GALVANIZED STANDARD WEIGHT 3 GA. "U" OR "W" WALL OR EQUAL. ALL REINFORCING SHALL BE LOCATED EVERY THIRD COURSE.
 - SUPPLY VERTICAL REINFORCING IN 4'-0" PLUS 30 DIAMETER BAR LAP LENGTHS.
 - DOWELS AS SHOWN SHALL MATCH SIZE AND NUMBER OF REINFORCING UNLESS OTHERWISE NOTED. HOOK INTO FOOTING 1'-0" MIN. AND LAP 30 DIAM. WITH MAIN STEEL.
 - WALL CONSTRUCTION FOR REINFORCING BARS AND INSULATION FILL SHALL BE 4'-0" LIFTS.
 - TYPE "M" MORTAR REQUIRED FOR BEARING WALLS BELOW GRADE. SEE ARCHITECTURAL PLANS FOR LOCATION AND DETAIL OF VERTICAL CONTROL JOINTS.
 - ALL STEEL BEAMS BEARING ON MASONRY SHALL HAVE 2 CORES BENCH FILLER WITH GROUT DIRECTLY BELOW THE BEARING POINT EXCEPT AS NOTED ON THE PLANS.
 - BOND BEAMS 8"-WIDE - USE 3-#5 CONT. BOND BEAMS 10"-OR LARGER - USE 3-#5 CONT. UNLESS NOTED OTHERWISE.
 - BRICK TIES:** (FOR MASONRY BACKUP) THERE SHALL BE A MINIMUM OF ONE BRICK TIE FOR EVERY 2 SQ. FT. OF WALL AREA. THESE SHALL BE SPACED AT A MAXIMUM OF 24" VERTICAL AND HORIZONTAL. TIES SHALL BE A MINIMUM OF 3/16" DIAMETER CORROSION RESISTANT WIRE. CORRUGATED GALVANIZED SHEET TIES ARE NOT ACCEPTABLE.
 - PRECAST STRESSED CONCRETE:**
 - PRECAST CONCRETE PLANKS, COLUMNS AND BEAMS SHALL BE DESIGNED AND CONSTRUCTED TO SAFELY SUSTAIN THE LIVE LOADS LISTED AND ALL SUPERIMPOSED DEAD LOADS IN ACCORDANCE WITH THE BUILDING CODE.
 - ALL CONNECTIONS SHALL BE DESIGNED, FABRICATED AND INSTALLED BY THE PRECAST SUPPLIER.
 - WELDING REQUIRED OF PLATES, BARS, INSERTS, ANCHORS ETC. SHALL CONFORM TO THE AWS'S "RECOMMENDED PRACTICES FOR WELDING REINFORCING STEEL, METAL CONNECTIONS IN REINFORCED CONCRETE CONSTRUCTION".
 - ALL REBAR SHALL HAVE FIRE RESISTIVE RATING IN ALL HEADERS AS PER APPROVED SHOP DRAWINGS. ALL HEADER ANGLES FOR THE PRECAST SHALL BE FURNISHED BY THE PRECASTER.
 - PRECAST AND CAST INTO THE PRECAST COLUMNS ALL ANCHOR BOLTS FOR THE STEEL COLUMNS AS DETAIL ON THE DRAWINGS.
 - PRECAST SUPPLIER SHALL FURNISH ALL PRECAST COLUMN CONNECTION DEVICES AND LAYOUT TO THE GENERAL CONTRACTOR FOR INSTALLATION IN THE FOOTINGS.
- STRUCTURAL STEEL:**
 - FABRICATION & ERECTION OF STRUCTURAL STEEL MEMBERS IS TO BE IN ACCORDANCE WITH A.I.S.C. CODE OF STANDARD PRACTICE.
 - ALL CONNECTIONS SHALL BE BOLTED OR WELDED AND SHALL DEVELOP THE FULL STRENGTH OF THE STRUCTURAL MEMBERS, UNLESS NOTED OTHERWISE. (MINIMUM OF 2 BOLTS PER CONNECTION).
 - ALL WELDING ELECTRODES SHALL BE E70X.
 - ALL WELDING SHALL BE BY QUALIFIED WELDERS AND SHALL CONFORM TO THE A.I.S.C.
 - FIELD CONNECTIONS ARE TO BE BOLTED. USE 3/4" DIAM. HIGH STRENGTH BOLTS AND NUTS (A325) UNLESS SHOWN OTHERWISE ON PLANS.
 - STEEL COLUMN BASE PLATES SHALL BE SIZE SHOWN ON PLAN WITH 3/4" DIAM. ANCHOR BOLTS (A307) AND 1" NON-SHRINK GROUT FOR UNIFORM BEARING.
 - UNLESS OTHERWISE NOTED, STRUCTURAL STEEL SUPPLIER IS TO PROVIDE COMPLETE CALCULATIONS AND/OR LOAD TEST DATA AT ALL ROOF OPENINGS. VERIFY SIZE AND LOCATION WITH MECHANICAL CONTRACTOR.
 - ALL STRUCTURAL STEEL AND MISCELLANEOUS METALS SHALL BE PRIME PAINTED WITH ONE COAT OF "PRIMER 899 PRIMER OR EQUAL" TOUCH UP ALL DISTURBED AREAS AFTER REPAIRS.
- STEEL JOIST:**
 - ALL STEEL JOISTS SHALL CONFORM TO THE REQUIREMENTS OF THE STEEL JOIST INSTITUTE STANDARD SPECIFICATIONS. JOIST FABRICATOR SHALL BE MEMBER OF THE SJI OR SHALL SUBMIT COMPLETE CALCULATIONS AND/OR LOAD TEST DATA CONFORMING TO SJI LOAD TEST TABLES.
 - BRIDGING FOR "I" SERIES STEEL JOISTS SHALL BE CONTINUOUS 3/4" DIAMETER OR EQUAL AT TOP AND BOTTOM OF JOISTS IN LENGTHS TO PERMIT LAPPING AT JOIST PANEL POINTS FOR WELDING. WELD BRIDGING TO CHORDS. SPACING OF BRIDGING IS NOTED ON DRAWINGS.
 - PROVIDE FULL AREA OF BOTTOM CHORD FOR JOISTS FRAMING INTO COLUMNS.
 - PROVIDE BOTTOM CHORD CEILING EXTENSIONS AS SHOWN ON ARCHITECTURAL DRAWINGS OR AS NOTED OTHERWISE.
 - HEADER ANGLES FOR STEEL JOISTS SHALL BE DESIGNED AND FURNISHED BY THE JOIST FABRICATOR AS NOTED ON THE DRAWINGS.
- STEEL ROOF DECK:**
 - STEEL DECK SHALL BE GALVANIZED.
 - END JOINTS OF DECK SHALL BE STAGGERED WITH 2" MINIMUM END LAP.
 - DECK SHALL BE 1 1/2" 22 GA INTERMEDIATE RIB DECK, TYPE 15.
 - METAL DECK IS TO BE WELDED AS THUS 12" O.C. OVER ALL INTERIOR SUPPORTS, 6" O.C. AT ALL EXTERIOR SUPPORTS AND 12" O.C. AT ALL EXTERIOR SIDE LAPS. WELD TO HAVE AN EFFECTED FUSION DIAMETER OF 5/8".
 - MAKE JOINTS OVER SUPPORTING MEMBER ONLY AND USE TRIPLE SPANS WHERE EVER POSSIBLE.
 - SELF-TAPPING SCREWS MAY BE USED WITH THE APPROVAL OF THE ARCHITECT. SCREW PATTERN SHALL DEVELOP EQUIVALENT DIAPHRAM VALUE AS WELD PATTERN.
- BACKFILLING:**
 - NO BACKFILLING AND COMPACTING OF EARTH SHALL BE PERMITTED AGAINST FOUNDATION WALLS UNLESS SUPPORTING SLABS HAVE BEEN POURED AND HAVE REACHED 75% OF THEIR DESIGN STRENGTH OR UNLESS ADEQUATE BRACING SUBMITTED FOR REVIEW IS PROVIDED.
 - BOTH SIDES OF FOUNDATION WALLS SHALL BE BACKFILLED SIMULTANEOUSLY SO AS TO PREVENT OVERTURNING OR LATERAL MOVEMENT OF WALLS.
- CONSTRUCTION AND CONTROL JOINTS:**
 - CONSTRUCTION JOINTS SHALL BE MADE AS DETAILED ON THE DRAWINGS.
 - A 20'-0" MAXIMUM SPACING OF CONTROL JOINTS MAY NOT BE USED WITHOUT THE WRITTEN APPROVAL OF THE ARCHITECT. SPACING MAY BE USED BY REQUEST OF OWNER IF MORE COMPLETE SHRINKAGE CRACK CONTROL IS DESIRED. CONTRACTOR TO VERIFY WITH OWNER.
 - CONSTRUCTION JOINTS IN CONCRETE FOUNDATION WALLS SHALL BE LOCATED SO NO SINGLE POUR IS LONGER THAN 40 FEET.
- EXPANSION BOLTS:**
 - ALL EXPANSION BOLTS SHALL BE EMBEDDED UNLESS OTHERWISE NOTED ON THE DRAWINGS. MINIMUM EMBEDMENT UNLESS OTHERWISE NOTED ON THE DRAWINGS.
- ERECTION:**
 - THE STRUCTURE SHALL BE ADEQUATELY BRACED AND SHORED DURING ERECTION AGAINST WIND AND ERECTION LOADS. STRUCTURAL MEMBERS ARE DESIGNED FOR "INPLACE" LOADS.
- MISCELLANEOUS:**
 - ALL ANCHOR BOLTS FOR MECHANICAL AND ELECTRICAL EQUIPMENT ARE FURNISHED AND LOCATED BY THE RESPECTIVE CONTRACTORS AND SET BY THE GENERAL CONTRACTOR EXCEPT WHERE THE OTHER CONTRACTORS FURNISH THEIR OWN CONCRETE PADS.
 - ALL PIPE SLEEVES ARE FURNISHED BY AND LOCATED BY THE MECHANICAL AND ELECTRICAL CONTRACTOR AND SET BY THE GENERAL CONTRACTOR.
 - THE GENERAL CONTRACTOR SHALL VERIFY ALL OPENINGS SIZES, PAD SIZES, AND LOCATIONS WITH THE RESPECTIVE CONTRACTORS.
 - ALL CORE DRILLING SHALL BE DONE BY THE MECHANICAL AND ELECTRICAL CONTRACTORS FOR THEIR OWN WORK UNDER THE SUPERVISION OF THE GENERAL CONTRACTOR. NO REINFORCING SHALL BE CUT. VERIFY LOCATION OF REINFORCING BEFORE CORE DRILLING. THERE SHALL NOT BE ANY CORE DRILLING THROUGH BEAMS OR COLUMNS. MAXIMUM CORE HOLE THROUGH SLABS SHALL BE PIPE DIAMETER PLUS 1".
- NEW CONSTRUCTION:**
 - THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. WHERE DISCREPANCIES OCCUR IT IS THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY THE ARCHITECT PRIOR TO CONSTRUCTION.
- NEW WORK IN CONJUNCTION WITH EXISTING CONSTRUCTION:**
 - THE CONTRACTOR SHALL VERIFY, BY FIELD CHECK, ALL SIZES, DIMENSIONS, AND LOCATIONS OF ALL EXISTING CONSTRUCTION PRIOR TO CONSTRUCTION WHICH ARE RELATIVE TO THE NEW CONSTRUCTION.
 - ALL DIMENSIONS INVOLVING NEW WORK TYING INTO OR GOVERNED BY EXISTING CONSTRUCTION SHALL BE FIELD CHECKED BY THE CONTRACTOR AND FURNISHED TO THE SUBCONTRACTOR PRIOR TO FABRICATION OF ANY WORK. THE VERIFIED DIMENSIONS SHALL APPEAR AND BE NOTED AS SUCH ON THE FIRST SHOP DRAWING SUBMITTED.
 - THE ENGINEER HAS MADE ASSUMPTIONS CONCERNING THE DIMENSIONS AND LOCATIONS OF ALL EXISTING CONSTRUCTION AND THESE ASSUMPTIONS ARE THAT THIS BUILDING WAS DESIGNED AND CONSTRUCTED IN CONJUNCTION WITH GOOD DESIGN AND CONSTRUCTION PRACTICES. THE CONTRACTOR SHALL TAKE EXTRAORDINARY CARE CONCERNING PRESERVATION OF EXISTING CONSTRUCTION WORK. FURTHER, HE SHALL AGREE TO ASSUME ALL RESPONSIBILITIES CONCERNING PRESERVATION OF EXISTING CONSTRUCTION.
 - THE CONTRACTOR SHALL NOTIFY THE ARCHITECT/ENGINEER IMMEDIATELY OF ANY DISCREPANCIES BETWEEN CONSTRUCTION DOCUMENTS AND ACTUAL FIELD CONDITIONS.
 - ALL HOLDS THROUGH EXISTING CONSTRUCTION SHALL BE CORE DRILLED OR SAW CUT.
- SHOP DRAWINGS:**
 - SHOP DRAWINGS, UNLESS OTHERWISE NOTED, SHALL BE SUBMITTED FOR REVIEW PRIOR TO FABRICATION ON REPRODUCIBLE TRANSPARENTS WITH ONE SET. SETS SHALL BE REPRODUCIBLE AND TRANSPARENTS AND PRINT IN A MAILING TUBE. TRANSPARENTS WHICH WILL NOT PRODUCE A LEGIBLE PRINT SHALL BE RETURNED FOR REWORK.
 - PRIOR TO SUBMITTAL, THE CONTRACTOR SHALL REVIEW THE SHOP DRAWINGS AND MAKE ANY CORRECTIONS REQUIRED. THE CONTRACTOR SHALL STAMP AND SIGN THE DRAWINGS THAT HE HAS REVIEWED THEM AND THAT THEY CORRECTLY REFLECT THE SHOP DRAWINGS SHALL BE FURNISHED FOR ALL STRUCTURAL COMPONENTS.

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I HEREBY CERTIFY THAT THIS PLAN, SPECIFICATION, OR REPORT WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION AND THAT I AM DULY REGISTERED AS AN ENGINEER UNDER THE LAWS OF THE STATE OF MINNESOTA.

DATE: 12-15-86 REG. NO. 133935

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